**9–1.** Determine the vertical displacement of joint *A*. Each bar is made of steel and has a cross-sectional area of 600 mm<sup>2</sup>. Take E = 200 GPa. Use the method of virtual work.



The virtual forces and real forces in each member are shown in Fig. a and b, respectively.

Member	n(kN)	N(kN)	L(m)	$nNL (kN^2 \cdot m)$
AB	1.25	6.25	2.50	19.531
AD	-0.75	-3.75	3	8.437
BD	-1.25	-6.25	2.50	19.531
BC	1.50	7.50	1.50	16.875
			Σ	64.375

$$1 \text{ kN} \cdot \Delta_{A_v} = \sum \frac{nNL}{AE} = \frac{64.375 \text{ kN}^2 \cdot \text{m}}{AE}$$

$$\Delta_{A_v} = \frac{64.375 \text{ kN} \cdot \text{m}}{AE}$$
  
=  $\frac{64.375(10^3) \text{ N} \cdot \text{m}}{\left[0.6(10^{-3}) \text{ m}^2\right] \left[200(10^9) \text{ N/m}^2\right]}$   
= 0.53646 (10<sup>-3</sup>) m  
= 0.536 mm  $\downarrow$ 







## **9–2.** Solve Prob. 9–1 using Castigliano's theorem.

Member	N(kN)	$rac{\partial N}{\partial P}$	N(P = 5kN)	$L(\mathbf{m})$	$N\left(\frac{\partial N}{\partial P}\right)L(\mathbf{kN}\cdot\mathbf{m})$
AB	1.25 P	1.25	6.25	2.5	19.531
AD	-0.750 P	-0.75	-3.75	3	8.437
BD	-1.25 P	-1.25	-6.25	2.5	19.531
BC	1.50 P	1.50	7.50	1.5	16.875
				Σ	64.375

$$\Delta_{A_v} = \sum N \left( \frac{\partial N}{\partial P} \right) \frac{L}{AE}$$
  
=  $\frac{64.375 \text{ kN} \cdot \text{m}}{AE}$   
=  $\frac{64 \cdot 375(10^3) \text{ N} \cdot \text{m}}{[0.6(10^{-3}) \text{ m}^2][200(10^9) \text{ N/m}^2]}$   
= 0.53646 (10<sup>-3</sup>) m  
= 0.536 mm  $\downarrow$ 

B 1.50P C 1.50P A + -0.750P P + 1.50P P + 1.50P P + 1.50P P + 1.50P P + 1.50P

C

2 m

**\*9–3.** Determine the vertical displacement of joint *B*. For each member  $A = 400 \text{ mm}^2$ , E = 200 GPa. Use the method of virtual work.

Member	п	Ν	L	nNL
AF	0	0	1.5	0
AE	-0.8333	-37.5	2.5	78.125
AB	0.6667	30.0	2.0	40.00
EF	0	0	2.0	0
EB	0.50	22.5	1.5	16.875
ED	-0.6667	-30.0	2.0	40.00
BC	0	0	2.0	0
BD	0.8333	37.5	2.5	78.125
CD	-0.5	-22.5	1.5	16.875
				$\Sigma = 270$

$$1 \cdot \Delta_{B_v} = \sum \frac{nNL}{AE}$$
$$\Delta_{B_v} = \frac{270(10^3)}{400(10^{-6})(200)(10^9)} = 3.375(10^{-3}) \,\mathrm{m} = 3.38 \,\mathrm{mm} \downarrow$$





IKN

Ans.



9–5.	Determine the vertical displacement of joint E. For
each	member $A = 400 \text{ mm}^2$ , $E = 200 \text{ GPa}$ . Use the method
of vir	tual work.

Member	п	Ν	L	nNL
AF	0	0	1.5	0
AE	-0.8333	-37.5	2.5	78.125
AB	0.6667	30.0	2.0	40.00
EF	0	0	2.0	0
EB	0.50	22.5	1.5	-16.875
ED	-0.6667	30.0	2.0	40.00
BC	0	0	2.0	0
BD	0.8333	37.5	2.5	78.125
CD	-0.5	-22.5	1.5	16.875
				$\Sigma = 236.25$

$$1 \cdot \Delta E_v = \sum \frac{nNL}{AE}$$
$$\Delta E_v = \frac{236.25(10^3)}{400(10^{-6})(200)(10^9)} = 2.95(10^{-3}) \,\mathrm{m} = 2.95 \,\mathrm{mm} \downarrow$$









AB	0.6667	10.0	4	26 667
RC	0.6667	10.0	4	26.667
AD	-0.8333	-12.5	5	52 083
RD	0.0555	15.0	3	0
	-0.8333	-12.5	5	52 083
CE CE	0.0505	27.5	3	41 25
	0.500	27.5	3	-1.25
	0	0		100.55
			$\Sigma$	198.75

$$1 \text{ kN} \cdot \Delta_{D_v} = \sum \frac{nNL}{AE} = \frac{198.75 \text{ kN}^2 \cdot \text{m}}{AE}$$
$$\Delta_{D_v} = \frac{198.75 \text{ kN} \cdot \text{m}}{AE} = \frac{199 \text{ kN} \cdot \text{m}}{AE} \quad \downarrow$$

311



**9-9.** Determine the vertical displacement of the truss at joint *F*. Assume all members are pin connected at their end points. Take A = 0.5 in<sup>2</sup> and  $E = 29(10^3)$  ksi for each member. Use the method of virtual work.



**9–11.** Determine the vertical displacement of joint *A*. The cross-sectional area of each member is indicated in the figure. Assume the members are pin connected at their end points.  $E = 29 (10)^3$  ksi. Use the method of virtual work.



The	virtual	force	and	real	force	in	each	member	are	shown	in	Fig.	а	and	b,
resp	ectively.														

Member	n(k)	N(k)	$L(\mathrm{ft})$	$nNL(k^2 \cdot ft)$
AB	-1.00	-7.00	4	28
BC	-1.00	-7.00	4	28
AD	$\sqrt{2}$	$7\sqrt{2}$	$4\sqrt{2}$	$56\sqrt{2}$
BD	-2.00	-14.00	4	112
CD	$\sqrt{2}$	$7\sqrt{2}$	$4\sqrt{2}$	$56\sqrt{2}$
CE	-1.00	-4.00	4	16
DE	0	0	4	0











Member	N(k)	$rac{\partial N}{\partial P}$	N(P = 7k)	L(ft)	$N\left(\frac{\partial N}{\partial P}\right)L(\mathbf{k}\cdot\mathbf{ft})$
AB	-P	-1	-7	4	28
BC	-P	-1	-7	4	28
AD	$\sqrt{2}P$	$\sqrt{2}$	$7\sqrt{2}$	$4\sqrt{2}$	$56\sqrt{2}$
BD	-2P	-2	-14	4	112
CD	$\sqrt{2}P$	$\sqrt{2}$	$7\sqrt{2}$	$4\sqrt{2}$	$56\sqrt{2}$
CE	-(P-3)	-1	-4	4	16
DE	0	0	0	4	0

$$\Delta_{A_v} = \sum N\left(\frac{\delta N}{\delta P}\right) \frac{L}{AE}$$
  
=  $\frac{(28 + 28 + 112 + 16) \text{ k} \cdot \text{ft}}{(3 \text{ in}^2)[29(10^3)\text{ k/m}^2]} + \frac{56\sqrt{2} + 56\sqrt{2} \text{ k}^2 \cdot \text{ft}}{(2 \text{ in}^2)[29(10^3)\text{ k/in}^2]}$   
= 0.004846 ft  $\left(\frac{12 \text{ in}}{1 \text{ ft}}\right) = 0.0582 \text{ in }\downarrow$ 





9-13. Determine the horizontal displacement of joint D. Assume the members are pin connected at their end points. AE is constant. Use the method of virtual work.



The virtual force and real force in each member are shown in Fig. a and b, respectively.

Member	n(k)	N(k)	$L(\mathrm{ft})$	$nNL(k^2 \cdot ft)$
AC	1.50	5.25	6	47.25
BC	-1.25	-6.25	10	78.125
BD	-0.75	-1.50	12	13.50
CD	1.25	2.50	10	31.25
			Σ	170.125

$$1\mathbf{k} \cdot \Delta_{D_h} = \sum \frac{nNL}{AE}$$

 $1\mathbf{k} \cdot \Delta_{D_h} = \frac{170.125 \, \mathbf{k}^2 \cdot \mathbf{ft}}{AE}$ 170 k • ft

$$\Delta_{D_h} = \frac{170 \,\mathrm{K} \cdot \mathrm{ft}}{AE} \quad \cdot$$





## 

Member	N(k)	$rac{\partial N}{\partial P}$	$N\left(P=2\mathbf{k}\right)$	$L(\mathrm{ft})$	$N\left(\frac{\partial N}{\partial P}\right)L(\mathbf{k}\cdot\mathbf{ft})$
AC	1.50P + 2.25	1.50	5.25	6	47.25
BC	-(1.25P + 3.75)	-1.25	-6.25	10	78.125
BD	-0.750P	-0.750	-1.50	12	13.5
CD	1.25P	1.25	2.50	10	31.25
				Σ	170.125

$$\Delta_{D_h} = \sum N \left( \frac{\delta N}{\delta P} \right) \frac{L}{AE}$$
$$= \frac{170.125 \text{ k} \cdot \text{ft}}{AE}$$
$$170 \text{ k} \cdot \text{ft}$$

$$=\frac{170 \text{ K}}{AE}$$

=



**9–15.** Determine the vertical displacement of joint *C* of the truss. Each member has a cross-sectional area of  $A = 300 \text{ mm}^2$ . E = 200 GPa. Use the method of virtual work.



The virtual and real forces in each member are shown in Fig. a and b respectively.

Member	n(kN)	N(kN)	$L(\mathbf{m})$	$nNL(kN^2 \cdot m)$
AB	0.6667	6.667	4	17.78
DE	0.6667	6.667	4	17.78
BC	1.333	9.333	4	49.78
CD	1.333	9.333	4	49.78
AH	-0.8333	-8.333	5	34.72
EF	-0.8333	-8.333	5	34.72
BH	0.5	5	3	7.50
DF	0.5	5	3	7.50
BG	-0.8333	-3.333	5	13.89
DG	-0.8333	-3.333	5	13.89
GH	-0.6667	-6.6667	4	17.78
FG	-0.6667	-6.6667	4	17.78
CG	1	4	3	12.00

 $\Sigma = 294.89$ 

$$1 \text{kN} \cdot \Delta_{C_v} = \sum \frac{nNL}{AE} = \frac{294.89 \text{ kN}^2 \cdot \text{m}}{AE}$$
$$\Delta_{C_v} = \frac{294.89 \text{ kN} \cdot \text{m}}{AE}$$
$$= \frac{294.89(10^3) \text{ N} \cdot \text{m}}{[0.3(10^{-3}) \text{ m}^2][200(10^9) \text{ N/m}^2]}$$
$$= 0.004914 \text{ m} = 4.91 \text{ mm} \quad \downarrow$$







Member	N(kN)	$rac{\partial N}{\partial P}$	N(P = 4  kN)	<i>L</i> (m)	$N\left(\frac{\partial N}{\partial P}\right)L\left(\mathbf{k}\cdot\mathbf{m}\right)$
AB	0.6667P + 4	0.6667	6.667	4	17.78
DE	0.6667P + 4	0.6667	6.667	4	17.78
BC	1.333P + 4	1.333	9.333	4	49.78
CD	1.333P + 4	1.333	9.333	4	49.78
AH	-(0.8333P+5)	-0.8333	-8.333	5	34.72
EF	-(0.8333P+5)	-0.8333	-8.333	5	34.72
BH	0.5P + 3	0.5	5	3	7.50
DF	0.5P + 3	0.5	5	3	7.50
BG	-0.8333P	-0.8333	-3.333	5	13.89
DG	-0.8333P	-0.8333	-3.333	5	13.89
GH	-(0.6667P + 4)	-0.6667	-6.667	4	17.78
FG	-(0.6667P + 4)	-0.6667	-6.667	4	17.78
CG	P	1	4	3	12.00
				Σ	294.89

$$\Delta_{C_v} = \sum N \left( \frac{\partial N}{\partial P} \right) \frac{L}{AE} = \frac{294.89 \text{ kN} \cdot \text{m}}{AE}$$
$$= \frac{294.89(10^3) \text{ N} \cdot \text{m}}{[0.3(10^{-3}) \text{ m}^2][200(10^9) \text{ N/m}^2]}$$
$$= 0.004914 \text{ m}$$









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**9–29.** Determine the slope and displacement at point C. Use the method of virtual work.  $E = 29(10^3)$  ksi, I = 800 in<sup>4</sup>.



Referring to the virtual moment functions indicated in Fig. a and b and the real moment function in Fig. c, we have

$$1\mathbf{k} \cdot \mathbf{ft} \cdot \theta_c = \int_0^L \frac{m_\theta M}{EI} dx = \int_0^{6 \text{ ft}} \frac{(-1)(-12)}{EI} dx_1 + \int_0^{6 \text{ ft}} \frac{(-1)[-(6x_2 + 12)]}{EI} dx_2$$
$$= \frac{252 \text{ k}^2 \cdot \text{ft}^3}{EI}$$
$$\theta_c = \frac{252 \text{ k} \cdot \text{ft}^2}{EI} = \frac{252(12^2) \text{ k} \cdot \text{in}^2}{[29(10^3) \text{ k/in}^2](800 \text{ in}^4)} = 0.00156 \text{ rad} \quad \forall \qquad \textbf{Ans.}$$

and

$$1\mathbf{k} \cdot \Delta_{C} = \int_{0}^{L} \frac{mM}{EI} \, dx = \int_{0}^{6 \, \text{ft}} \frac{(-x_{1})(-12)}{EI} \, dx_{1} + \int_{0}^{6 \, \text{ft}} \frac{([-(x_{2} + 6)][-(6x_{2} + 12)]}{EI} \, dx_{2}$$
$$= \frac{1944 \, \text{k}^{2} \cdot \text{ft}^{3}}{EI}$$
$$\Delta_{C} = \frac{1944 \, \text{k} \cdot \text{ft}^{3}}{EI} = \frac{1944(12^{3}) \, \text{k} \cdot \text{in}^{3}}{[29(10^{3}) \, \text{k}/\text{in}^{2}](800 \, \text{in}^{4})} = 0.415 \, \text{in} \quad \downarrow \qquad \text{Ans.}$$

$$(m_{\theta})_{z} = -1k \cdot ft$$

$$(m_{\theta})_{z} = -(x_{z} + 6) \quad (m_{z} = -x_{z})$$

$$(m_{\theta})_{z} = -(x_{z} + 6) \quad (m_{z} = -x_{z})$$

$$(m_{\theta})_{z} = -(x_{z} + 6) \quad (m_{z} = -x_{z})$$

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$$(m_{\theta})_{z} = -(x_{z} + 6) \quad (m_{z} = -x_{z})$$

$$(m_{\theta})_{z} = -(x_{z} + 6) \quad (m_{z} = -x_{z})$$

$$(m_{\theta})_{z} = -(x_{z} + 6) \quad (m_{z} = -x_{z})$$





**9–31.** Determine the displacement and slope at point *C* of the cantilever beam. The moment of inertia of each segment is indicated in the figure. Take  $E = 29(10^3)$  ksi. Use the principle of virtual work.



Referring to the virtual moment functions indicated in Fig. a and b and the real moment function in Fig. c, we have

$$1\mathbf{k} \cdot \mathbf{ft} \cdot \theta_{c} = \int_{0}^{L} \frac{m_{0}M}{EI} dx = \int_{0}^{3} \frac{\mathbf{ft}(-1)(-50)}{EI_{BC}} dx_{1} + \int_{0}^{6} \frac{\mathbf{ft}(-1)(-50)}{EI_{AB}} dx_{1}$$

$$1\mathbf{k} \cdot \mathbf{ft} \cdot \theta_{c} = \frac{150 \,\mathbf{k}^{2} \cdot \mathbf{ft}^{3}}{EI_{BC}} + \frac{300 \,\mathbf{k}^{2} \cdot \mathbf{ft}^{3}}{EI_{AB}}$$

$$\theta_{c} = \frac{150 \,\mathbf{k} \cdot \mathbf{ft}^{2}}{EI_{BC}} + \frac{300 \,\mathbf{k} \cdot \mathbf{ft}^{2}}{EI_{AB}}$$

$$= \frac{150(12^{2}) \,\mathbf{k} \cdot \mathbf{in}^{2}}{[29(10^{3}) \,\mathbf{k}/\mathbf{in}^{2}](200 \,\mathbf{in}^{4})} + \frac{300(12^{2}) \,\mathbf{k} \cdot \mathbf{in}^{2}}{[29(10^{3}) \,\mathbf{k}/\mathbf{in}^{2}](500 \,\mathbf{in}^{4})}$$

$$= 0.00670 \,\mathbf{rad} \quad \forall$$

And

$$1 \mathbf{k} \cdot \Delta_{C} = \int_{0}^{L} \frac{mM}{EI} dx = \int_{0}^{3 \text{ ft}} \frac{-x_{1}(-50)}{EI_{BC}} dx_{1} + \int_{0}^{6 \text{ ft}} \frac{-(x_{2}+3)(-50)}{EI_{AB}} dx_{2}$$

$$1 \mathbf{k} \cdot \Delta_{C} = \frac{225 \,\mathbf{k}^{2} \cdot \text{ft}^{3}}{EI_{BC}} + \frac{1800 \,\mathbf{k}^{2} \cdot \text{ft}^{3}}{EI_{AB}}$$

$$\Delta_{C} = \frac{225 \,\mathbf{k} \cdot \text{ft}^{3}}{EI_{BC}} + \frac{1800 \,\mathbf{k}^{2} \cdot \text{ft}^{3}}{EI_{AB}}$$

$$= \frac{225(12^{3}) \,\mathbf{k} \cdot \text{in}^{3}}{[29(10^{3}) \,\mathbf{k}/\text{in}^{2}](200 \,\text{in}^{4})} + \frac{1800(12^{3}) \,\mathbf{k} \cdot \text{in}^{3}}{[29(10^{3}) \,\mathbf{k}/\text{in}^{2}](500 \,\text{in}^{4})} = 0.282 \,\text{in} \downarrow$$



\*9-32. Solve Prob. 9-31 using Castigliano's theorem.



For the slope, the moment functions are shown in Fig. a. Here,  $\frac{\partial M_1}{\partial M'} = \frac{\partial M_2}{\partial M'} = -1$ . Also, set  $M' = 50 \text{ k} \cdot \text{ft}$ , then  $M_1 = M_2 = -50 \text{ k} \cdot \text{ft}$ .

Thus,

$$\theta_{C} = \int_{0}^{L} M\left(\frac{\partial M}{\partial M'}\right) \frac{dx}{EI} = \int_{0}^{3 \text{ ft}} \frac{-50(-1)dx}{EI_{BC}} + \int_{0}^{6 \text{ ft}} \frac{-50(-1)dx}{EI_{AB}}$$
$$= \frac{150 \text{ k} \cdot \text{ft}^{2}}{EI_{BC}} + \frac{300 \text{ k} \cdot \text{ft}^{2}}{EI_{AB}}$$
$$= \frac{150(12^{2}) \text{ k} \cdot \text{in}^{2}}{[29(10^{3} \text{ k/in}^{2})](200 \text{ in}^{4})} + \frac{300(12^{2}) \text{ k} \cdot \text{in}^{2}}{[29(10^{3}) \text{ k/in}^{2}](500 \text{ in}^{4})}$$
$$= 0.00670 \quad \forall$$

For the displacement, the moment functions are shown in Fig, b. Here,  $\frac{\partial M_1}{\partial P} = -x_1$ and  $\frac{\partial M_2}{\partial P} = -(x_2 + 3)$ . Also, set P = 0, then  $M_1 = M_2 = -50$  k  $\cdot$  ft. Thus,  $\Delta_C = \int_0^L M\left(\frac{\partial M}{\partial P}\right) \frac{dx}{EI} = \int_0^{3 \text{ ft}} \frac{(-50)(-x)dx}{EI_{BC}} + \int_0^{6 \text{ ft}} \frac{(-50)[-(x_2+3)]dx}{EI_{AB}}$  $= \frac{225 \text{ k} \cdot \text{ft}^3}{EI_{BC}} + \frac{1800 \text{ k} \cdot \text{ft}^3}{EI_{AB}}$  $= \frac{225(12^3) \text{ k} \cdot \text{in}^3}{[29(10^3) \text{ k/in}^2](200 \text{ in}^4)} + \frac{1800(12^3) \text{ k} \cdot \text{in}^3}{[29(10^3) \text{ k/in}^2](500 \text{ in}^4)}$  $= 0.282 \text{ in } \downarrow$ 







**9–34.** Solve Prob. 9–33 using Castigliano's theorem.



For the slope, the moment function is shown in Fig. *a*. Here,  $\frac{\partial M}{\partial M'} = -1$ . Also, set M' = 0, then  $M = -(150x^2 + 400x) \,\mathrm{N} \cdot \mathrm{m}$ . Thus,

$$\theta_B = \int_0^L M\left(\frac{\partial M}{\partial M'}\right) \frac{dx}{EI} = \int_0^{3 \text{ m}} \frac{-(150x^2 + 400x)(-1)}{EI} dx$$
$$= \frac{3150 \text{ N} \cdot \text{m}^2}{EI} \quad \forall \qquad \text{Ans.}$$

For the displacement, the moment function is shown in Fig. b. Here,  $\frac{\partial M}{\partial P} = -x$ . Also, set P = 400 N, then  $M = (400x + 150x^2)$  N · m. Thus,

$$\Delta_B = \int_0^L M\left(\frac{\partial M}{\partial P}\right) \frac{dx}{EI} = \int_0^{3 \text{ m}} \frac{-(400x + 150x^2)(-x)}{EI} dx$$
$$= \frac{6637.5 \text{ N} \cdot \text{m}^3}{EI} \downarrow$$
Ans



(a)



9-35. Determine the slope and displacement at point B. 4 k/ftAssume the support at A is a pin and C is a roller. Take  $E = 29(10^3)$  ksi, I = 300 in<sup>4</sup>. Use the method of virtual work. B - 10 ft 5 ft Referring to the virtual moment functions shown in Fig. a and b and the  $(M_0) = -0.06667 \chi_z$ real moment function shown in Fig. c,  $1 \mathbf{k} \cdot \mathbf{ft} \cdot \theta_B = \int_0^L \frac{m_\theta M}{EI} dx = \int_0^{10 \text{ ft}} \frac{(0.06667x_1)(30x_1 - 2x_1^2)dx_1}{EI}$  $(m_{\rho}) = 0.06667X$ 1 K·ft  $+ \int_{0}^{5 \text{ ft}} \frac{(-0.06667x_2)(30x_2 - 2x_2^2)}{EI} dx_2$  $1 \mathbf{k} \cdot \mathbf{ft} \cdot \boldsymbol{\theta}_B = \frac{270.83 \mathbf{k}^2 \cdot \mathbf{ft}^3}{FI}$ 10ft 0.0667K 0.0667K  $\theta_B = \frac{270.83 \text{ k} \cdot \text{ft}^2}{EI} = \frac{270.83(12^2) \text{ k} \cdot \text{in}^2}{[29(10^3) \text{ k/in}^2](300 \text{ in}^4)} = 0.00448 \text{ rad} \measuredangle \text{ Ans.}$ (a) And  $1 \mathbf{k} \cdot \Delta_B = \int_0^L \frac{mM}{EI} dx = \int_0^{10 \text{ ft}} \frac{(0.3333x_1)(30x_1 - 2x_1^2)dx_1}{EI}$ 1ĸ m,=0.3333 m,=0.6667X +  $\int_{0}^{5 \text{ ft}} \frac{(0.6667x_2)(30x_2 - 2x_2^2)dx_2}{EI}$  $1 \mathbf{k} \cdot \Delta_B = \frac{2291.67 \mathbf{k} \cdot \mathbf{ft}^3}{EI}$ х,  $\Delta_B = \frac{2291.67 \text{ k} \cdot \text{ft}^3}{EI} = \frac{2291.67(12^3) \text{ k} \cdot \text{in}^3}{[29(10^3) \text{ k/in}^2](300 \text{ in}^4)} = 0.455 \text{ in } \downarrow \text{ Ans.}$ 10ft 5ft 0.3333 K 0.6667K *(b)* M,=30x,-2x;  $M_2 = 30\chi_2 - 2\chi_2^2$ 10ft 30 K (C)



 $\frac{\partial M_1}{\partial p} = 0.3333x_1 \text{ and } \frac{\partial M_2}{\partial P} = 0.6667x_2. \text{ Also, set } P = 0, \text{ then}$  $M_1 = (30x_1 - 2x_1^2) \text{ k} \cdot \text{ft and } M_2 = (30x_2 - 2x_2^2) \text{ k} \cdot \text{ft. Thus}$ 

$$\Delta_B = \int_0^L M\left(\frac{\partial M}{\partial P}\right) \frac{dx}{EI} = \int_0^{10 \text{ ft}} \frac{30x_1 - 2x_1^2(0.3333x_1)dx_1}{EI} + \int_0^{5 \text{ ft}} \frac{(30x_2 - 2x_2^2)(0.6667x_2)dx_2}{EI} = \frac{2291.67 \text{ k} \cdot \text{ft}^3}{EI} = \frac{2291.67(12^3) \text{ k} \cdot \text{in}^3}{[29(10^3)\text{ k}/\text{in}^2](300 \text{ in}^4)} = 0.455 \text{ in } \downarrow$$







**9-37.** Determine the slope and displacement at point *B*. Assume the support at *A* is a pin and *C* is a roller. Account for the additional strain energy due to shear. Take  $E = 29(10^3)$  ksi, I = 300 in<sup>4</sup>,  $G = 12(10^3)$  ksi, and assume *AB* has a cross-sectional area of A = 7.50 in<sup>2</sup>. Use the method of virtual work.



The virtual shear and moment functions are shown in Fig. a and b and the real shear and moment functions are shown in Fig. c.

$$1 \text{ k} \cdot \text{ft} \cdot \theta_{B} = \int_{0}^{L} \frac{m_{\theta}M}{EI} dx + \int_{0}^{L} \text{k} \left(\frac{\nu V}{GA}\right) dx$$
  
$$= \int_{0}^{10 \text{ ft}} \frac{0.06667x_{1}(30x_{1} - 2x_{1}^{2})}{EI} dx_{1} + \int_{0}^{10 \text{ ft}} 1 \left[\frac{0.06667(30 - 4x_{1})}{GA}\right] dx_{1}$$
  
$$+ \int_{0}^{5 \text{ ft}} \frac{(-0.06667x_{2}(30x_{2} - 2x_{2}^{2})}{EI} dx_{2} + \int_{0}^{5 \text{ ft}} 1 \left[\frac{0.06667(4x_{2} - 30)}{GA}\right] dx_{2}$$
  
$$= \frac{270.83 \text{ k}^{2} \cdot \text{ft}^{3}}{EI} + 0$$
  
$$\theta_{B} = \frac{270.83 \text{ k} \cdot \text{ft}^{2}}{EI} = \frac{270.83(12^{2}) \text{ k} \cdot \text{in}^{2}}{[29(10^{3}) \text{ k/in}^{2}(300 \text{ in}^{4})]} = 0.00448 \text{ rad} \quad \measuredangle \qquad \text{Ans.}$$

And

$$1 \mathbf{k} \cdot \Delta_{B} = \int_{0}^{L} \frac{mM}{EI} dx + \int_{0}^{L} \mathbf{k} \left(\frac{\nu V}{GA}\right) dx$$

$$= \int_{0}^{10 \text{ ft}} \frac{(0.3333x_{1})(30x_{1} - 2x_{1}^{2})}{EI} dx_{1} + \int_{0}^{10 \text{ ft}} \left[\frac{0.3333(30 - 4x_{1})}{GA}\right] dx_{1}$$

$$+ \int_{0}^{5 \text{ ft}} \frac{(0.6667x_{2})(30x_{2} - 2x_{2}^{2})}{EI} dx_{2} + \int_{0}^{5 \text{ ft}} \left[\frac{(-0.6667)(4x_{2} - 30)}{GA}\right] dx_{2}$$

$$= \frac{2291.67 \text{ k}^{2} \cdot \text{ft}^{3}}{EI} + \frac{100 \text{ k}^{2} \cdot \text{ft}}{GA}$$

$$\Delta_{B} = \frac{2291.67 \text{ k} \cdot \text{ft}^{3}}{EI} + \frac{100 \text{ k} \cdot \text{ft}}{GA}$$

$$= \frac{2291.67(12^{3}) \text{ k} \cdot \text{in}^{3}}{[29(10^{3}) \text{ k/in}^{2}](300 \text{ in}^{4})} + \frac{100(12) \text{ k} \cdot \text{in}}{[12(10^{3}) \text{ k/in}^{2}](7.50 \text{ in}^{2})}$$

$$= 0.469 \text{ in } \downarrow$$

## 9-37. Continued





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**9–38.** Determine the displacement of point *C*. Use the method of virtual work. *EI* is constant.

Referring to the virtual and real moment functions shown in Fig. a and b, respectively,

$$1 \cdot \Delta_C = \int_0^L \frac{mM}{EI} dx = 2 \int_0^{\frac{L}{2}} \frac{\left(\frac{1}{2}\right) \left(\frac{w_0 L}{4} x - \frac{w_0}{3L} x^3\right)}{EI} dx$$
$$\Delta_C = \frac{w_0 L^4}{120 EI} \quad \downarrow$$

Ans.

W

 $\frac{L}{2}$ 

С

 $\frac{L}{2}$ 





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**\*9–40.** Determine the slope and displacement at point *A*. Assume C is pinned. Use the principle of virtual work. EI is constant.



Referring to the virtual moment functions shown in Fig. a and b and the real moment functions in Fig. c, we have

$$1 \text{ kN} \cdot \text{m} \cdot \theta_A = \int_0^L \frac{m_{\theta} M}{EI} dx = \int_0^{3 \text{ m}} \frac{(-1)(-0.3333x_1^3)}{EI} dx_1 + \int_0^{3 \text{ m}} \frac{(0.3333x_2)(6x_2 - 3x_2^2)}{EI} dx_2$$

 $1 \text{ kN} \cdot \text{m} \cdot \theta_A = \frac{9 \text{ kN}^2 \cdot \text{m}^3}{EI}$ 

 $\theta_A = \frac{9 \,\mathrm{kN} \cdot \mathrm{m}^2}{EI} \quad \varUpsilon$ 

And

Ans.

1 KN

Ans.



EI



(a)



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9-42. Determine the displacement at point D. Use the 8 k principle of virtual work. EI is constant. 3 k/ft B|D -4 ft — -4 ft -- 4 ft -4 ft Referring to the virtual and real moment functions shown in Fig. a and b, respectively,  $1 \mathbf{k} \cdot \Delta_D = \int_0^L \frac{mM}{EI} dx = \int_0^{4 \text{ ft}} \frac{(-0.5x_1)(-12x_1)}{EI} dx_1 + \int_0^{4 \text{ ft}} \frac{[-0.5(x_2 + 4)][-(20x_2 + 48)]}{EI} dx_2$ +  $2\int_{0}^{4 \text{ ft}} \frac{(-0.5x_3)(12x_3 - 1.50x_3^2)}{EI} dx_3$  $1 \mathbf{k} \cdot \Delta_D = \frac{1397.33 \mathbf{k}^2 \cdot \mathbf{ft}^3}{EI}$  $\Delta_D = \frac{1397 \,\mathrm{k} \cdot \mathrm{ft}^3}{EI} \downarrow$ Ans.  $m_2 = -0.5(\chi_2 + 4)$ 0.5K 1kM,=-0.5X m3=0.5X M,=0.5) 4ft 0.5K 0.5K (a) 8k IZK M3=12×3-1.50×  $M_{2} = -(20X_{2} + 48)$ 128k (b)<sup>12k</sup> 4ft 4ft 12K

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**9–46.** The L-shaped frame is made from two segments, each of length L and flexural stiffness EI. If it is subjected to the uniform distributed load, determine the horizontal displacement of the end C. Use the method of virtual work.

$$1 \cdot \Delta_{C_h} = \int_0^L \frac{mM}{EI} dx$$
$$\Delta_{C_h} = \frac{l}{EI} \left[ \int_0^L (1x_1) \left( \frac{wx_1^2}{2} \right) dx_1 + \int_0^L (1L) \left( \frac{wL^2}{2} \right) dx_2 \right]$$
$$= \frac{5wL^4}{8EI}$$



A

A

**9–47.** The L-shaped frame is made from two segments, each of length L and flexural stiffness EI. If it is subjected to the uniform distributed load, determine the vertical displacement of point B. Use the method of virtual work.





## 9–49. Continued



**9–51.** Determine the vertical deflection at *C*. The cross-sectional area and moment of inertia of each segment is shown in the figure. Take E = 200 GPa. Assume *A* is a fixed support. Use the method of virtual work.



$$(\Delta_C)_v = \int_0^L \frac{mM}{EI} dx = \int_0^3 \frac{(3-x)(50)(10^3)dx}{EI_{AB}} + 0$$
$$= \frac{[150(10^3)x - 25(10^3)x^2]_0^3}{EI_{AB}}$$
$$= \frac{225(10^3)}{200(10^9)(400)(10^6)(10^{-12})}$$
$$= 2.81 \text{ mm } \downarrow$$

**\*9–52.** Solve Prob. 9–51, including the effect of shear and axial strain energy.



See Prob. 9–51 for the effect of bending.

$$U = \sum \frac{nNL}{AE} + \int_0^L K\left(\frac{\nu V}{GA}\right) dx$$

Note that each term is zero since n and N or v and V do not occur simultaneously in each member. Hence,

$$(\Delta_C)_v = 2.81 \ mm \downarrow$$
 Ans



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